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A MULTIDOMAIN DISCRETIZATION OF THE RICHARDS EQUATION IN LAYERED SOIL

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Preprint A /02/2013



**FACHBEREICH MATHEMATIK UND INFORMATIK
SERIE A • MATHEMATIK**

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ABSTRACT. We consider the Richards equation on a domain that is decomposed into nonoverlapping layers, i.e., the decomposition has no cross points. We assume that the saturation and permeability functions are space-independent on each subdomain. Kirchhoff transformation of each subdomain problem separately then leads to a set of semi-linear equations, which can each be solved efficiently using monotone multigrid. The transformed subdomain problems are coupled by nonlinear continuity and flux conditions. This nonlinear coupled problem can be solved using substructuring methods like the Dirichlet–Neumann or Robin iteration. We give several numerical examples showing the discretization error, the solver robustness under variations of the soil parameters and a hydrological example with four soil layers and surface water.

1. INTRODUCTION

The Richards equation [7, 22, 33] is a well-established model for saturated–unsaturated groundwater flow. In its most general form it can be written as

$$(1) \quad n(x)\theta(x,p)_t + \operatorname{div} \mathbf{v}(x,p) = f, \quad \mathbf{v}(x,p) = -K_h(x)kr(x,\theta(x,p))\nabla(p-z)$$

for $(x,t) \in \Omega \times (0,T)$. The first equation represents mass conservation with a source term f , while the second one is a generalized Darcy law [23] for the water flux \mathbf{v} . The flux \mathbf{v} includes a gravity term ∇z , where z is the third component of x , and the z -axis points downwards. The dependencies on $x \in \Omega \subset \mathbb{R}^3$ indicate the heterogeneities of the soil. The parameters $n : \Omega \rightarrow (0,1)$ and $K_h : \Omega \rightarrow \mathbb{R}^+$ are the porosity and the hydraulic conductivity, respectively. The behaviour of the porous medium is further captured by the soil water retention relationship $\theta(x,\cdot) : \mathbb{R} \rightarrow [\theta_m(x),\theta_M(x)]$, and the relative permeability $kr(x,\cdot) : [\theta_m(x),\theta_M(x)] \rightarrow [0,1]$. For any $x \in \Omega$, the saturation θ of the soil stays between the residual and the maximal saturation $\theta_m(x), \theta_M(x) \in [0,1]$. Furthermore, it is an increasing function of the water pressure p (in case of $p > 0$) or the capillary pressure (in case of $p \leq 0$) for any fixed $x \in \Omega$, while, again for fixed x , the function $\theta \mapsto kr(x,\theta)$ is increasing with respect to θ . The unit of the physical pressure p is given in meters (of a water column).

We use concrete representations according to Brooks and Corey [20] of the parameter functions $\theta(x,\cdot)$ and $kr(x,\cdot)$ given in terms of two soil parameters which depend on x : the bubbling pressure $p_b(x) < 0$ and the pore size distribution factor $\lambda(x) > 0$. By the property $kr(x,\cdot) > 0$ with $kr(x,\theta(p)) \rightarrow 0$ for $p \rightarrow -\infty$, these functions induce a degeneracy in the elliptic–parabolic equation (1). Moreover,

1991 *Mathematics Subject Classification.* MSC (2000): 65N30, 65N55, 76S05.

This work was supported by the BMBF–Programm “Mathematik für Innovationen in Industrie und Dienstleistungen”.

seepage phenomena lead to nonlinear nonsmooth outflow boundary conditions of Signorini's type at the interface between soil and air. These features leads to difficulties both in the analytical and numerical treatment of this equation. In fact, existence and uniqueness results are restricted to spatially homogeneous parameter functions $\theta(\cdot)$ and $kr(\cdot)$ [1, 2, 3].

Big slopes of the parameter functions may have a negative effect on Newton's method applied to the corresponding finite dimensional algebraic system, e.g., in case of very coarse sand with small bubbling pressures. Therefore, in contrast to concentrating on the discretization first and on the algebraic solution later (as in [4, 26, 34, 37, 38]), we recently derived a Kirchhoff finite element discretization which is solver-friendly in the sense that iterative solution of the resulting spatial problems by monotone multigrid methods is robust with respect to soil parameters and exhibits similar convergence speed as in the saturated linear case [16]. However, this approach heavily relies on the reformulation of the given quasilinear spatial problems in terms of convex minimization, based on Kirchhoff transformation. Therefore, it is restricted to homogeneous soil, i.e., spatially homogeneous parameter functions $\theta(\cdot)$ and $kr(\cdot)$.

The purpose of this paper is to extend the Kirchhoff finite element approach to the heterogeneous case, i.e., to spatially varying parameter functions $\theta(x, \cdot)$ and $kr(x, \cdot)$. In hydrologically realistic cases, heterogeneous soil is often built of layers of different homogeneous soil types. Therefore, we restrict ourselves to situations in which the dependency of the parameter functions $\theta(x, \cdot)$ and $kr(x, \cdot)$ on $x \in \Omega$ is not completely arbitrary, but in which we have a nonoverlapping decomposition of Ω into subdomains Ω_i , $i = 1, \dots, n$, without cross points (see [17] for possible extensions in this direction) and x -independent $\theta_i(\cdot)$ and $kr_i(\cdot)$ for each Ω_i . Under this assumption, we can derive a generalized multidomain formulation of the time-discretized Richards equation (1): After Kirchhoff transformation in the subdomains Ω_i , continuity of physical pressure and fluxes across the interfaces transforms into nonlinear transmission conditions across the interfaces. Finite element discretization and node-wise application of the inverse Kirchhoff discretization then provides a novel multidomain discretization. In each subdomain Ω_i it can be interpreted as a standard finite element discretization with inexact quadrature [16], or alternatively as a finite element method with a modified metric [?]. Using a model problem with known exact solution, we numerically found optimal convergence rates both in the H^1 - and the L^2 -norm, whereas theoretical justification is still open.

By construction, our multidomain discretization is solver-friendly in the sense that existing nonlinear domain decomposition methods [12, 14, 15] can be applied in combination with fast and robust subdomain solvers. Concerning mesh dependence, extensive numerical studies revealed surprising similarities of nonlinear versions of Dirichlet–Neumann and Robin iterations to their linear counterpart [15]. Here, we concentrate on the numerical assessment of robustness with respect to soil parameters. The ill-conditioned, inverse Kirchhoff transformation enters the iteration via the interface conditions and thus, in contrast to the homogeneous case, is no longer separated from the solution process. Nevertheless, both for Dirichlet–Neumann and Robin methods, we found surprising robustness of the convergence rates with respect to the whole range of practically relevant values of pore size distribution and bubbling pressure.

Numerical approximation of outflow boundary conditions of Signorini-type often relies on regularization [35, 36]. As our approach is based on convexity rather than on smoothness, outflow boundary conditions can be easily incorporated in terms of variational inequalities. No regularization and thus no regularization parameters are needed. This is illustrated by a numerical example with real-life data taken from [8]: We simulate surface water seeping into unsaturated, layered soil and seeping out again once the soil is saturated due to subsurface flow. Surface water is described by a simple compartment model. Coupling is performed by pressure continuity and mass conservation across the interface using Signorini-type outflow boundary conditions. This additional nonsmooth nonlinearity turns out not to cause any problems concerning efficiency and reliability of the overall solution process. Extension to more complicated situations, like, e.g., flooding, is the subject of current research.

The paper is organized as follows. In Section 2 we formally state the multidomain formulation of the Richards equation both in the physical pressure p and after Kirchhoff transformation. Section 3 gives discretizations both in time and space. To make this paper self-contained, Section 4 recalls substructuring algorithms for the solution of the coupled algebraic systems, concentrating on the Dirichlet–Neumann and the Robin method. Finally, Section 5 is devoted to numerical examples. First, we assess the discretization error of our multidomain discretization (Subsection 5.1). As mesh-dependence of Dirichlet–Neumann and Robin iterations has been investigated elsewhere [15], we focus on robustness with respect to variations of the soil parameters in Subsection 5.2. Finally we present the simulation of a surface / groundwater seepage process with saturated–unsaturated layered soil and realistic hydrological data (Subsection 5.3).

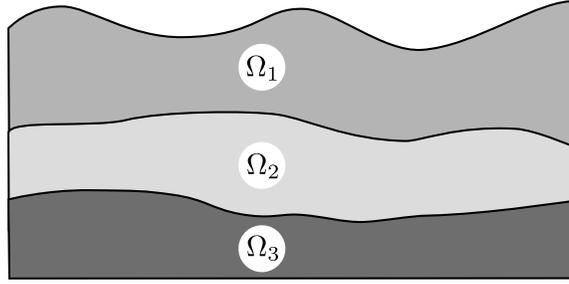
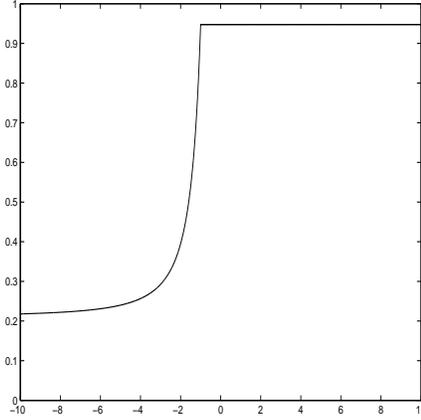
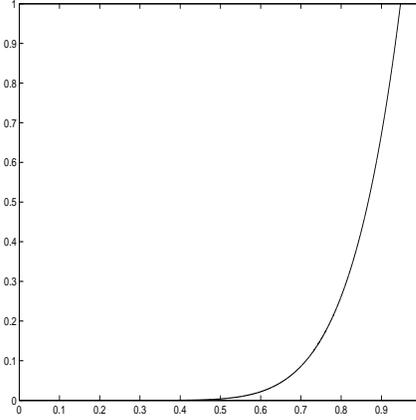
2. RICHARDS EQUATION IN LAYERED SOIL

2.1. Multidomain formulation in physical variables. Let $\Omega \subset \mathbb{R}^d$, $d = 1, 2, 3$, be a bounded Lipschitz domain decomposed into non-overlapping subdomains Ω_i , $i = 1, \dots, n$, $n \in \mathbb{N}$, which also have Lipschitz boundaries. Then the interfaces $\Gamma_{ij} := \overline{\Omega}_i \cap \overline{\Omega}_j$, $1 \leq i, j \leq n$, $i \neq j$, are Lipschitz continuous, too, and the normal \mathbf{n}_{ij} on Γ_{ij} in the direction of Ω_j exists almost everywhere. We assume that we do not have cross points, i.e., $\Gamma_{ij} \cap \Gamma_{i'j'} = \emptyset$ for $i \neq i'$ or $j \neq j'$, so that the subdomains form layers in Ω as shown in Figure 1, ordered by $1, \dots, n$ with Ω_1 on top and Ω_n on the bottom. Therefore, only the Γ_{ij} with $i, j = 1, \dots, n$ and $j = i \pm 1$ are nonempty.

Each layer Ω_i is associated to a set of soil parameters n_i , $K_{h,i}$, $\theta_{m,i}$, $\theta_{M,i}$, $p_{b,i}$ and λ_i , as described in the introduction. The saturation–pressure relations $\theta_i(\cdot)$ and the relative permeability–saturation functions $kr_i(\cdot)$ associated to the subdomain Ω_i are x -independent and given by the formulas

$$(2) \quad \theta_i(p) = \begin{cases} \theta_{m,i} + (\theta_{M,i} - \theta_{m,i}) \left(\frac{p}{p_{b,i}} \right)^{-\lambda_i} & \text{for } p \leq p_{b,i} \\ \theta_{M,i} & \text{for } p \geq p_{b,i} \end{cases}$$

$$(3) \quad kr_i(\theta) = \left(\frac{\theta - \theta_{m,i}}{\theta_{M,i} - \theta_{m,i}} \right)^{3 + \frac{2}{\lambda_i}}, \quad \theta \in [\theta_{m,i}, \theta_{M,i}],$$

FIGURE 1. Decomposition of Ω into layers containing different soil typesFIGURE 2. $p \mapsto \theta_i(p)$ FIGURE 3. $\theta \mapsto kr_i(\theta)$

given by Brooks and Corey [20] and Burdine [21]. See Figures 2 and 3 for typical shapes of these functions. We call this setting *layered soil* from now on.

Now, we formulate our problem for saturated–unsaturated groundwater flow in this type of heterogeneous porous media: Find \tilde{p} on $\Omega \times (0, T)$ satisfying suitable initial and boundary conditions to be specified below, such that with $\tilde{p}_i := \tilde{p}|_{\Omega_i \times (0, T)}$ and abbreviating $\tilde{f}_i := f|_{\Omega_i \times (0, T)}$ we have for all $i = 1, \dots, n$

$$(4) \quad n_i \theta_i(\tilde{p}_i)_t - \operatorname{div} (K_{h,i} kr_i(\theta_i(\tilde{p}_i)) \nabla(\tilde{p}_i - z)) = \tilde{f}_i \quad \text{in } \Omega_i \times (0, T),$$

and for all $i = 1, \dots, n-1, j = i+1$, the interface conditions

$$(5) \quad \tilde{p}_i = \tilde{p}_j$$

$$(6) \quad K_{h,i} kr_i(\theta_i(\tilde{p}_i)) \nabla(\tilde{p}_i - z) \cdot \mathbf{n}_{ij} = K_{h,j} kr_j(\theta_j(\tilde{p}_j)) \nabla(\tilde{p}_j - z) \cdot \mathbf{n}_{ij}$$

hold on $\Gamma_{ij} \times (0, T)$. One can show that the weak form of the global Richards equation (1) is equivalent to the weak form of the multidomain problem (4)–(6). The former entails continuity of the pressure (5) as well as of the water flux, i.e., mass conservation (6), compare [12] and see also Section 3.1.2.

2.2. Boundary conditions. On Lipschitz submanifolds γ_D and γ_N of $\partial\Omega$ we consider Dirichlet and Neumann boundary conditions $\tilde{p} = \tilde{p}_D$ and $\tilde{\mathbf{v}} \cdot \mathbf{n} = f_N$, respectively, with admissible and compatible functions p_D and f_N . Here, the water flux

$\tilde{\mathbf{v}}$ is the dual variable corresponding to the primal variable \tilde{p} by (1) and \mathbf{n} is the outward normal on $\partial\Omega$. Robin boundary conditions of different type [12, 18] can also be included. In this paper, we only apply them on the interfaces in connection with the Robin method, see Subsection 4.2.

Our formulation can easily accommodate another important nontrivial boundary condition. On the interface between a porous medium and air, one can sometimes observe seepage faces. There, the water pressure cannot exceed zero, water can flow out of the porous medium only if the water pressure is zero and there is no flow otherwise. Mathematically, such a behaviour on a Lipschitz submanifold γ_S of $\partial\Omega$ can be described by the Signorini-type condition [12, 16, 35]

$$\tilde{p} \leq 0, \quad \tilde{\mathbf{v}} \cdot \mathbf{n} \geq 0, \quad \tilde{p} \cdot (\tilde{\mathbf{v}} \cdot \mathbf{n}) = 0 \quad \text{a.e. on } \gamma_S \times (0, T).$$

A priori, it is unknown where outflow and where noflow occurs, i.e., we actually have a free boundary value problem. We do not allow nonempty intersections $\overline{\gamma_S} \cap \Gamma_{ij}$, because a substructuring formulation is lacking a rigorous justification in this case [12].

2.3. Multidomain formulation in generalized variables via local Kirchhoff transformation. We now turn the quasilinear equation (4) on each subdomain into a corresponding semi-linear equation as follows. To each of the subdomain problems (4) we apply a Kirchhoff transformation (see, e.g., [3, 25]), defined by

$$\kappa_i : \tilde{p}_i \mapsto \tilde{u}_i := \int_0^{\tilde{p}_i} kr_i(\theta_i(q)) dq,$$

where the new variable \tilde{u}_i is called *generalized pressure*. The chain rule provides

$$(7) \quad \nabla \tilde{u}_i = \kappa_i'(\tilde{p}_i) \nabla \tilde{p}_i = kr_i(\theta_i(\tilde{p}_i)) \nabla \tilde{p}_i$$

and the saturation as a function of \tilde{u}_i shall be denoted by

$$(8) \quad M_i(\tilde{u}_i) := \theta_i(\kappa_i^{-1}(\tilde{u}_i)).$$

Figures 4 and 5 show the typical behaviour of the inverse transformation κ_i^{-1} and of M_i . Note that by superposition, the real functions κ_i and κ_i^{-1} can also be regarded as superposition operators acting on spaces of real-valued functions. Denoting the unit vector in the direction of gravity by $e_z := \nabla z$, the transformed subdomain problems and interface conditions (4)–(6) read

$$(9) \quad n_i M_i(\tilde{u}_i)_t - \operatorname{div} (K_{h,i}(\nabla \tilde{u}_i - kr_i(M_i(\tilde{u}_i))e_z)) = \tilde{f}_i \quad \text{in } \Omega_i \times (0, T)$$

for $i = 1, \dots, n$, and for all $i = 1, \dots, n-1$, $j = i+1$, the interface conditions

$$(10) \quad \kappa_i^{-1} \tilde{u}_i = \kappa_j^{-1} \tilde{u}_j$$

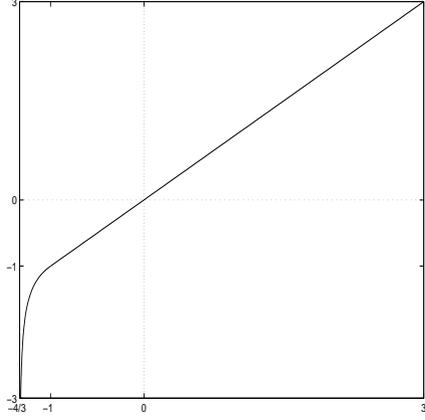
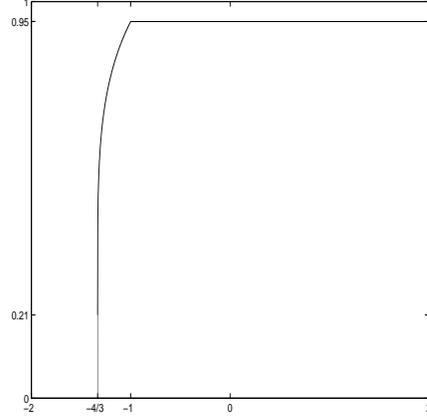
$$(11) \quad K_{h,i}(\nabla \tilde{u}_i - kr_i(M_i(\tilde{u}_i))e_z) \cdot \mathbf{n}_{ij} = K_{h,j}(\nabla \tilde{u}_j - kr_j(M_j(\tilde{u}_j))e_z) \cdot \mathbf{n}_{ij}$$

hold on $\Gamma_{ij} \times (0, T)$. The advantage of the Kirchhoff transformation for the homogeneous cases is that the subproblems (9) are semilinear equations whereas the untransformed problems (4) are quasilinear. Since $\kappa_i(0) = 0$ for all $i = 1, \dots, n$ and the water flux is

$$(12) \quad \tilde{\mathbf{v}} = -K_{h,i}(\nabla \tilde{u}_i - kr_i(M_i(\tilde{u}_i))e_z)$$

in the transformed variable \tilde{u} on $\Omega \times (0, T)$ given by $\tilde{u}|_{\Omega_i \times (0, T)} := \tilde{u}_i$ for $i = 1, \dots, n$, the transformed Signorini-type boundary condition reads

$$(13) \quad \tilde{u} \leq 0, \quad \tilde{\mathbf{v}} \cdot \mathbf{n} \geq 0, \quad \tilde{u} \cdot (\tilde{\mathbf{v}} \cdot \mathbf{n}) = 0 \quad \text{a.e. on } \gamma_S \times (0, T).$$

FIGURE 4. $u \mapsto \kappa_i^{-1}(u)$ FIGURE 5. $u \mapsto M_i(u)$

Dirichlet and Neumann boundary conditions are transformed accordingly. In particular, transformed homogeneous boundary conditions are again homogeneous. We refer to [9] and [8, Sec. 1.5.4] for a detailed discussion of the conditions needed to prove equivalence of untransformed and transformed problems.

3. MULTIDOMAIN DISCRETIZATION FOR THE RICHARDS EQUATION

3.1. Time-discrete physical and generalized multidomain formulation.

3.1.1. *Strong form.* Let $0 = t_0 < t_1 < \dots < t_N = T$ be a partition of $[0, T]$, k the time step number, and $\tau_k := t_k - t_{k-1}$ for $k \in \{1, \dots, N\}$ the time step size. Let \bar{p}_i be the discrete solution on the i -th subdomain for the time t_{k-1} . We discretize (4) implicitly in the main part of the spatial derivative and explicitly in the convective, i.e., the gravitational part. Therefore, at time t_k the time-discrete spatial multidomain problem in physical variables is to find p_i on Ω_i such that

$$(14) \quad n_i \theta_i(p_i) - \tau_k \operatorname{div} (K_{h,i} k r_i(\theta_i(p_i)) \nabla p_i) = f_i \quad \text{in } \Omega_i$$

holds for $i = 1, \dots, n$ with coupling conditions

$$(15) \quad p_i = p_j \quad \text{on } \Gamma_{ij}$$

$$(16) \quad \tau_k K_{h,i} k r_i(\theta_i(p_i)) \nabla p_i \cdot \mathbf{n}_{ij} - f_{ij} = \tau_k K_{h,j} k r_j(\theta_j(p_j)) \nabla p_j \cdot \mathbf{n}_{ij} - f_{ji} \quad \text{on } \Gamma_{ij}$$

for $i = 1, \dots, n-1$, $j = i+1$. Setting $\bar{f}_i(\cdot) := \tilde{f}_i(\cdot, t_k)$ in Ω_i , we abbreviate

$$(17) \quad f_i := \tau_k \bar{f}_i + n_i \theta_i(\bar{p}_i) - \tau_k K_{h,i} k r_i(\theta_i(\bar{p}_i))_z$$

as well as

$$f_{ij} := \tau_k K_{h,i} k r_i(\theta_i(\bar{p}_i)) e_z \cdot \mathbf{n}_{ij} \quad \text{and} \quad f_{ji} := \tau_k K_{h,j} k r_j(\theta_j(\bar{p}_j)) e_z \cdot \mathbf{n}_{ij}.$$

By $u_i := \kappa_i(p_i)$, $\bar{u}_i := \kappa_i(\bar{p}_i)$ and replacing $\theta_i(p_i)$ and $\theta_i(\bar{p}_i)$ by $M_i(u_i)$ and $M_i(\bar{u}_i)$, respectively, in these formulas with the same set of indices i and j , the corresponding time-discrete version of the Kirchhoff-transformed multidomain formulation (9)–(11) reads

$$(18) \quad n_i M_i(u_i) - \tau_k \operatorname{div} (K_{h,i} \nabla u_i) = f_i \quad \text{in } \Omega_i$$

with coupling conditions

$$(19) \quad \kappa_i^{-1} u_i = \kappa_j^{-1} u_j \quad \text{on } \Gamma_{ij}$$

$$(20) \quad \tau_k K_{h,i} \nabla u_i \cdot \mathbf{n}_{ij} - f_{ij} = \tau_k K_{h,j} \nabla u_j \cdot \mathbf{n}_{ij} - f_{ji} \quad \text{on } \Gamma_{ij}.$$

The implicit–explicit time discretization of the water flux (12) at the time t_k reads

$$\mathbf{v} = -K_{h,i} (\nabla u_i - kr_i(M_i(\bar{u}_i)) e_z) \quad \text{in } \Omega_i$$

so that the time-discrete Signorini-type boundary condition (13) can be written as

$$u \leq 0, \quad \mathbf{v} \cdot \mathbf{n} \geq 0, \quad u \cdot (\mathbf{v} \cdot \mathbf{n}) = 0 \quad \text{on } \gamma_S,$$

where u is defined by $u := u_i$ in Ω_i .

3.1.2. Weak form. We now turn to the weak formulation of (14)–(16) and its Kirchhoff–transformed variant (18)–(20). For simplicity and without loss of generality we restrict ourselves to homogeneous Dirichlet boundary conditions on $\partial\Omega$ in this and in the next section. We use the Sobolev spaces $H^1(\Omega_i)$ and $H_0^1(\Omega_i)$ (the latter with homogeneous Dirichlet boundary conditions) as well as the trace space $H^{1/2}(\Gamma)$ and the trace operator $\text{tr}_\Gamma : H^1(\Omega_i) \rightarrow H^{1/2}(\Gamma)$, $i = 1, \dots, n$, for a Lipschitz manifold $\Gamma \subset \partial\Omega_i$, consult, e.g., [19]. Additionally, we introduce the spaces

$$(21) \quad V_i := \{v_i \in H^1(\Omega_i) : \text{tr}_{\partial\Omega \cap \partial\Omega_i} v_i = 0\}$$

for $i = 1, \dots, n$ and

$$\Lambda_{ij} := \{\eta \in H^{1/2}(\Gamma_{ij}) : \eta = \text{tr}_{\Gamma_{ij}} v_i \text{ for a } v_i \in V_i\} = H_0^{1/2}(\Gamma_{ij})$$

for $|i - j| = 1$, $i, j = 1, \dots, n$. For the same set of indices we also choose linear continuous extension operators

$$R_{ij} : \Lambda_{ij} \rightarrow V_i \quad \text{and} \quad R_{ji} : \Lambda_{ij} \rightarrow V_j$$

that are right-inverses to the trace operators

$$\text{tr}_{\Gamma_{ij}} : V_i \rightarrow \Lambda_{ij} \quad \text{and} \quad \text{tr}_{\Gamma_{ji}} : V_j \rightarrow \Lambda_{ij}$$

from both sides of Γ_{ij} . Despite of the non-uniqueness $\Lambda_{ij} = \Lambda_{ji}$ for $i = 1, \dots, n-1$, $j = i+1$, which we also have for the interfaces and operators above, we use this set of indices because it will simplify the notation for the algorithms in Section 4. By $(\cdot, \cdot)_{\Omega_i}$ we abbreviate the L^2 -scalar product for scalar- and vector-valued functions on Ω_i . We define the bi-form

$$(22) \quad b_i(w_i, v_i) := (\tau_k K_{h,i} kr_i(w_i) \nabla w_i, \nabla v_i)_{\Omega_i} \quad \forall w_i, v_i \in H^1(\Omega_i)$$

as well as the bilinear form

$$a_i(w_i, v_i) := (\tau_k K_{h,i} \nabla w_i, \nabla v_i)_{\Omega_i} \quad \forall w_i, v_i \in H^1(\Omega_i)$$

and the linear form

$$(23) \quad \ell_i(v_i) := (\bar{f}_i + n_i \theta_i(\bar{p}_i), v_i)_{\Omega_i} + (\tau_k K_{h,i} kr_i(\theta_i(\bar{p}_i)) e_z, \nabla v_i)_{\Omega_i} \quad \forall v_i \in H^1(\Omega_i).$$

Then in a weak formulation the multidomain problem (14)–(16) in physical variables reads as follows: Find $p_i \in V_i$, $i = 1, \dots, n$, satisfying the subdomain equations

$$(24) \quad (n_i \theta_i(p_i), v_i)_{\Omega_i} + b_i(p_i, v_i) = \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i), \quad i = 1, \dots, n,$$

and the weak coupling conditions

$$(25) \quad \text{tr}_{\Gamma_{ij}} p_i = \text{tr}_{\Gamma_{ji}} p_j \quad \text{in } \Lambda_{ij}$$

$$(26) \quad (n_i \theta_i(p_i), R_{ij}\mu)_{\Omega_i} + b_i(p_i, R_{ij}\mu) - \ell_i(R_{ij}\mu) = \\ - (n_j \theta_j(p_j), R_{ji}\mu)_{\Omega_j} - b_j(p_j, R_{ji}\mu) + \ell_j(R_{ji}\mu) \quad \forall \mu \in \Lambda_{ij},$$

where (25) and (26) hold for all $i = 1, \dots, n-1$ with $j = i+1$. By defining $p := p_i$ on Ω_i , $i = 1, \dots, n$, and using (25) one obtains $p \in H_0^1(\Omega)$. Note that the application of Green's formula in the weak sense (see, e.g., [19]) allows to replace the functional $\ell_i(v_i)$ by the integral $(f_i, v_i)_{\Omega_i}$ with f_i from (17) in (24), but not analogously in (26).

A weak form of (18)–(20) is now obtained by Kirchhoff transformation of (24)–(26), in which we rewrite (23) as

$$(27) \quad \ell_i(v_i) = (\bar{f}_i + n_i M_i(\bar{u}_i), v_i)_{\Omega_i} + (\tau_k K_{h,i} kr_i(M_i(\bar{u}_i))e_z, \nabla v_i)_{\Omega_i} \quad \forall v_i \in H^1(\Omega_i)$$

and assume that $p_i \in V_i \Leftrightarrow u_i \in V_i$ holds with $u_i = \kappa_i(p_i)$. Thus our *time-discrete, generalized multidomain formulation* reads: Find $u_i \in V_i$, $i = 1, \dots, n$, satisfying the subdomain equations

$$(28) \quad (n_i M_i(u_i), v_i)_{\Omega_i} + a_i(u_i, v_i) = \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i), \quad i = 1, \dots, n,$$

and the weak coupling conditions

$$(29) \quad \kappa_i^{-1} \operatorname{tr}_{\Gamma_{ij}} u_i = \kappa_j^{-1} \operatorname{tr}_{\Gamma_{ji}} u_j \quad \text{in } \Lambda_{ij}$$

$$(30) \quad (n_i M_i(u_i), R_{ij}\mu)_{\Omega_i} + a_i(u_i, R_{ij}\mu) - \ell_i(R_{ij}\mu) = \\ - (n_j M_j(u_j), R_{ji}\mu)_{\Omega_j} - a_j(u_j, R_{ji}\mu) + \ell_j(R_{ji}\mu) \quad \forall \mu \in \Lambda_{ij},$$

where (29) and (30) hold for all $i = 1, \dots, n-1$ with $j = i+1$. Note that in general, the subdomain functions u_i on Ω_i , $i = 1, \dots, n$, do not give rise to a global function $u \in H_0^1(\Omega)$, since in case of $\kappa_i^{-1} \neq \kappa_j^{-1}$ the coupling condition (29) induces a discontinuity of the transformed variables across the interfaces.

Equivalence $p_i \in V_i \Leftrightarrow u_i \in V_i$ holds if and only if κ_i and κ_i^{-1} are (for $d = 1$ locally) Lipschitz continuous, see [?]. This condition guarantees familiar solution spaces for u_i , $i = 1, \dots, n$, and trace spaces in (29), but it is not satisfied for the Brooks–Corey parametrization. In this case, the inverse Kirchhoff transformations κ_i^{-1} , $i = 1, \dots, n$, even have a singularity, see Figure 4. Nevertheless, they can be used successfully in numerical simulations, see Section 5.

Proposition 3.1. *If $\theta_i : \mathbb{R} \rightarrow \mathbb{R}$ and $kr_i : \theta_i(\mathbb{R}) \rightarrow \mathbb{R}$ are increasing and bounded and $kr_i(\cdot) \geq c$ for $a > 0$, $i = 1, \dots, n$, then the multidomain problem (24)–(26) in physical variables is equivalent to the multidomain problem (28)–(30) in generalized variables.*

If the assumptions in the proposition are only satisfied for $c = 0$, then one needs to require $u_i \in \kappa_i(V_i)$ in (24)–(26) in order to maintain equivalence. For details and generalizations concerning this equivalence result we refer to [9] or [8, Sec. 1.5.4]. Note, e.g., that (25) \Leftrightarrow (29) requires κ_i^{-1} and $\operatorname{tr}_{\Gamma_{ij}}$ to commute. A substructuring result which states the equivalence of a weak global problem and its corresponding domain decomposition problem such as (14)–(16) in weak form is derived in [12], for the more general case involving Signorini-type boundary conditions. That result provides a mathematical justification of the transmission conditions (25) and (26). For a generalization of these transmission conditions to domain decompositions with cross points see [17].

3.1.3. *Well-posedness of local subproblems.* Now, we shortly outline the solution theory for the subproblems (28) equipped with homogeneous Dirichlet boundary conditions on all of $\partial\Omega_i$. Here, the Kirchhoff transformation and our special time discretization allow us to use convex analysis.

We require $n_i, K_{h,i} \in L^\infty(\Omega_i)$, $n_i \geq 0$ and $K_{h,i} \geq c > 0$. Furthermore, let $M_i : \mathbb{R} \rightarrow \mathbb{R}$ and $kr_i : [0, 1] \rightarrow \mathbb{R}$ be increasing and bounded and $\bar{f}_i \in L^2(\Omega_i)$ in (27). Then, with a convex primitive Φ_i of M_i , the functional

$$(31) \quad \phi_i : v_i \mapsto \int_{\Omega_i} n_i \Phi_i(v_i) dx \quad \forall v_i \in H_0^1(\Omega_i)$$

is proper, convex and lower semicontinuous. Furthermore, the quadratic functional

$$\mathcal{J}_i : v_i \mapsto \frac{1}{2}a_i(v_i, v_i) - \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i)$$

is coercive and continuous. Consequently, there exists a unique minimizer of $\phi_i + \mathcal{J}_i$ in $H_0^1(\Omega_i)$, see, e.g., [24]. Now, finding $u_i \in H_0^1(\Omega_i)$ satisfying (28) is equivalent to determining this minimizer because (28) is just the Euler–Lagrange equation of $\phi_i + \mathcal{J}_i$.

We point out that this line of thought can be generalized to cover quite general boundary conditions including Robin and Signorini-type conditions. The former lead to another convex functional on $H^1(\Omega_i)$ as long as $\kappa_i^{-1} : \mathbb{R} \rightarrow \mathbb{R}$ is Lipschitz continuous [12]. The latter provide a constraint to the set of solutions, so that the functional is to be minimized on a convex subset of $H_0^1(\Omega_i)$. In this case the variational equality (28) becomes an elliptic variational inequality of the first kind. Analogously, Brooks–Corey functions, which lead to an ill-posed κ_i^{-1} with a singularity, can be considered, too, since they just add another constraint to the convex set. For further details on the solution of the time-discretized Kirchhoff–transformed Richards equation in homogeneous soil with general boundary conditions consult [16] and [8, Sec. 2.3, Thm. 3.4.2].

3.2. Fully discrete generalized multidomain formulation. In this section we describe a spatial discretization of the generalized multidomain formulation (28)–(30) based on Lagrange finite elements of first order. The resulting discrete problems suggest nonlinear extensions of well-known substructuring methods because fast and robust solvers for the corresponding local convex subproblems are available [16]. Special attention needs to be turned to the discretization of the second scalar product occurring in (27), since this expression contains an explicitly time-discretized convective term that has to be treated by an upwinding technique.

3.2.1. *Discrete generalized multidomain problem.* We assume that Ω is polygonal and that we have a triangulation \mathcal{T}_l on Ω with the set of interior vertices \mathcal{N}_l . Here, the subscript l is the discretization index. We choose Lagrange finite elements of first order on the grid, and hence \mathcal{N}_l is also the set of all Lagrange nodes q which determine the Lagrange basis $\{\lambda_q\}_{q \in \mathcal{N}_l}$ of the corresponding finite element space $\mathcal{S}_l \subset H_0^1(\Omega)$. Further we assume that all interfaces Γ_{ij} , $i = 1, \dots, n-1$, $j = i+1$, are resolved by the grid, and we denote by $\mathcal{N}_{l,ij}$ the set of all interior vertices located on the interface Γ_{ij} for $i, j = 1, \dots, n$, $j = i \pm 1$. The finite element space $\mathcal{S}_{l,ij} \subset \Lambda_{ij}$ induced by these nodes is defined accordingly. (As in Section 3.1.2 we allow for non-uniqueness of the notation which will be helpful in Section 4.) We call $\mathcal{N}_{l,i}$ the subset of all interior vertices of Ω_i and $\mathcal{S}_{l,i}^0 \subset H_0^1(\Omega_i)$ the finite element

space spanned by the Lagrange basis corresponding to these nodes. Furthermore, $\mathcal{S}_{l,i} \subset V_i$ is the finite element space spanned by the basis functions corresponding to all nodes in $\mathcal{N}_l \cap \bar{\Omega}_i$, cf. (21). We introduce the weights

$$h_{q,i} := \int_{\Omega_i} n_i \lambda_q dx \quad \forall q \in \mathcal{N}_l$$

and define the extension operators

$$R_{l,ij} : \mathcal{S}_{l,ij} \rightarrow \mathcal{S}_{l,i} \quad \text{and} \quad R_{l,ji} : \mathcal{S}_{l,ij} \rightarrow \mathcal{S}_{l,j}$$

by zero extension to all nodes in $\mathcal{N}_{l,i}$ and $\mathcal{N}_{l,j}$, respectively. By $\ell_{l,i}(v_i)$ we denote an upwind finite element discretization of $\ell_i(v_i)$ to be specified in Subsection 3.2.2.

Altogether, we are now in a position to give our spatial discretization of (18)–(20). It reads: Find $u_{l,i} \in \mathcal{S}_{l,i}$, $i = 1, \dots, n$, satisfying

$$(32) \quad \sum_{q \in \mathcal{N}_{l,i}} M_i(u_{l,i}(q)) v_{l,i}(q) h_{q,i} + a_i(u_{l,i}, v_{l,i}) = \ell_{l,i}(v_{l,i}) \quad \forall v_{l,i} \in \mathcal{S}_{l,i}$$

with discrete coupling conditions

$$(33) \quad \kappa_i^{-1} u_{l,i}(q) = \kappa_j^{-1} u_{l,j}(q) \quad \forall q \in \mathcal{N}_{l,ij}$$

$$(34) \quad \sum_{q \in \mathcal{N}_{l,ij}} M_i(u_{l,i}(q)) \mu_{l,ij}(q) h_{q,i} + a_i(u_{l,i}, R_{l,ij} \mu_{l,ij}) - \ell_{l,i}(R_{l,ij} \mu_{l,ij}) = \\ - \sum_{q \in \mathcal{N}_{l,ij}} M_j(u_{l,j}(q)) \mu_{l,ij}(q) h_{q,j} - a_j(u_{l,j}, R_{l,ji} \mu_{l,ij}) + \ell_{l,j}(R_{l,ji} \mu_{l,ij})$$

$$\forall \mu_{l,ij} \in \mathcal{S}_{l,ij}$$

where (33) and (34) hold for all $i = 1, \dots, n-1$ with $j = i+1$.

In general, as in the continuous and in the time-discrete case, the discrete functions $u_{l,i}$, $i = 1, \dots, n$, do not give rise to a function $u \in \mathcal{S}_l$ but rather to a function that is two-valued on each interface Γ_{ij} , $i = 1, \dots, n-1$, $j = i+1$. However, the coupling condition (33) enforces the continuity of the physical variables across the interfaces and can be rewritten accordingly. One can also rewrite (32) and (34) in physical variables so that the coupled problem (32)–(34) is equivalent to a finite element discretization of the time-discrete physical problem with a special quadrature for the integrals involving nonlinear terms. For details we refer to [16].

3.2.2. Upwind discretization of gravity. Finally, we turn to the discretization of $\ell_i(v_i)$ in (28) and the corresponding terms in (30). By (27) the functional ℓ_i is given as a sum of two scalar products, the first of which is discretized by a quadrature rule with a sufficiently high order. The second scalar product in $\ell_i(v_i)$ reads

$$(35) \quad \left(\tau_k K_{h,i} kr_i(M_i(\bar{u}_i)), \frac{\partial}{\partial z} v_i \right)_{\Omega_i}.$$

If we only consider the time derivative and the spatial derivative $kr_i(M_i(\bar{u}_i))_z$ on the left hand side of the Richards equation (9), we obtain a nonlinear conservation law, so that for stability reasons it is necessary to apply an upwind discretization to $kr_i(M_i(\bar{u}_i))_z$, consult [28]. Note that (35) is obtained from $kr_i(M_i(\bar{u}_i))_z$ by Green's formula and explicit time discretization. If we discretize (35) with finite elements $v_i \in \mathcal{S}_{l,i}$, interpolate $kr_i(M_i(\bar{u}_i))$ in $\mathcal{S}_{l,i}$ and assume $K_{h,i} = \text{const.}$, this gives central differences of $-kr_i(M_i(\bar{u}_i))$ in z -direction up to a constant in case of a regular mesh with vertical grid lines parallel to the direction of gravity. It is well

known that by adding or subtracting an *artificial viscosity* term, i.e., diffusion or, equivalently, central differences for the second derivative, one can obtain one-sided differences. Therefore, with a certain small weighting factor related to the mesh size, we subtract a diffusion matrix in z -direction to the convection matrix obtained by finite element discretization of (35) in order to achieve upwinding of $-kr_i(M_i(\bar{u}_i))$ in z -direction within the finite element framework. The CFL condition guarantees a monotone scheme, i.e., numerical stability, in the linear case if time step sizes are chosen sufficiently small. Further details can be found in [8, Sec. 4.2.1] and in [16].

3.2.3. Minimization formulation for discrete local subproblems. The sums in (32) and (34) represent a discretization of the corresponding integrals in (28) and (30) by Lagrange interpolation of the product $M_i(u_i) v_i$, respectively. They can be regarded as a lumping of the L^2 -scalar product and represent the Fréchet derivative of a discretization of the convex functional ϕ_i in (31) in which $\Phi_i(v_i)$ is interpolated by Lagrange functions. Therefore, with homogeneous Dirichlet boundary conditions for instance, as in the continuous case, each finite dimensional subproblem (32) for $i = 1, \dots, n$ is equivalent to a uniquely solvable minimization problem for a convex functional on $\mathcal{S}_{l,i}$. This result can be extended to the use of Brooks–Corey parameter functions and quite general Dirichlet, Neumann, Robin and Signorini-type boundary conditions. Furthermore, for homogeneous problems, convergence of the discrete solutions to the continuous solution for decreasing mesh size can be shown in the Brooks–Corey case and for general boundary conditions [8, 16, 18].

3.3. Multidomain discretization in physical variables via local inverse discrete Kirchhoff transformation. Observe that one can rewrite the discrete interface condition (33) more similar to its continuous counterpart (29) if one considers the Lagrange interpolation operator

$$I_{\mathcal{S}_{l,ij}} : \Lambda_{ij} \cap C(\Gamma_{ij}) \rightarrow \mathcal{S}_{l,ij}$$

defined by $(I_{\mathcal{S}_{l,ij}}\mu)(q) = \mu(q)$ for all $q \in \mathcal{N}_{l,ij}$ and all relevant indices i, j, l . Then (33) reads

$$(36) \quad I_{\mathcal{S}_{l,ij}} \kappa_i^{-1}(u_{l,i}|_{\Gamma_{ij}}) = I_{\mathcal{S}_{l,ij}} \kappa_j^{-1}(u_{l,j}|_{\Gamma_{ij}}).$$

Analogously we define the interpolation operator

$$I_{\mathcal{S}_{l,i}} : V_i \cap C(\bar{\Omega}_i) \rightarrow \mathcal{S}_{l,i}$$

given by $(I_{\mathcal{S}_{l,i}}v)(q) = v(q)$ for all $q \in \mathcal{N}_{l,i}$ and all relevant indices i, l .

Definition 3.2. We call $I_{\mathcal{S}_{l,i}} \kappa_i : \mathcal{S}_{l,i} \rightarrow \mathcal{S}_{l,i}$ discrete Kirchhoff transformation on $\mathcal{S}_{l,i}$, and $p_l \in \mathcal{S}_l$ given by

$$(37) \quad p_{l,i} = I_{\mathcal{S}_{l,i}} \kappa_i^{-1} u_{l,i}, \quad i = 1, \dots, n,$$

discrete physical pressure associated with the discrete generalized multidomain problem (32)–(34).

The inverse discrete Kirchhoff transformation on $\mathcal{S}_{l,i}$ is $I_{\mathcal{S}_{l,i}} \kappa_i^{-1}$, and $I_{\mathcal{S}_{l,ij}} \kappa_i^{-1}$ as well as $I_{\mathcal{S}_{l,ij}} \kappa_j^{-1}$ in (36) can be regarded as inverse discrete Kirchhoff transformations on $\mathcal{S}_{l,ij}$ corresponding to Ω_i and Ω_j , respectively.

Now, just as the continuous multidomain problem (28)–(30) in generalized variables arose from Kirchhoff transformation of the corresponding problem (24)–(26) in physical pressure, one can obtain a discrete multidomain problem in physical

pressure by inverse discrete Kirchhoff transformation of the corresponding discrete problem (32) – (34) in generalized variables. This finally amounts to a new discretization of the physical multidomain problem (24)–(26).

In fact, by definition of M_i , $i = 1, \dots, n$, in (8), the terms in the sums as well as in the linear functionals $\ell_{l,i}$, $i = 1, \dots, n$, cf. (28) and (30), can be easily rewritten in discrete physical pressure. Discrete Kirchhoff transformation of (29) follows directly from (37). The reformulation of the bilinear forms $a_i(\cdot, \cdot)$ in terms of $p_{l,i}$, however, is not straightforward. In fact, just as $a_i(\cdot, \cdot)$ arose from $b_i(\cdot, \cdot)$ by the chain rule (7) applied to the Kirchhoff transformation in the continuous setting, a kind of discrete chain rule involving the discrete Kirchhoff transformation is needed now. Such a discrete chain rule was derived in [16] using the mean value theorem. It reads as follows.

Lemma 3.3 (Discrete chain rule). *Let $\theta_i : \mathbb{R} \rightarrow \mathbb{R}$ and $kr_i : \theta_i(\mathbb{R}) \rightarrow \mathbb{R}$ be increasing and continuous and $kr_i(\cdot) > 0$ for $i = 1, \dots, n$. Let $t \in \mathcal{T}_i$ be a simplex of the grid on Ω_i given by edges e_k , $k = 1, \dots, d$, that meet in a point. Furthermore, let $p_{l,i}$ and $u_{l,i} = I_{\mathcal{S}_{l,i}} \kappa_i(p_{l,i})$ in $\mathcal{S}_{l,i}$ be given for $i = 1, \dots, n$. Then there exist points x_k , $k = 1, \dots, d$, each located on the edge e_k , and depending on κ_i and $p_{l,i}$ such that the discrete chain rule*

$$\nabla u_{l,i} = D_t(p_{l,i}) \nabla p_{l,i} \quad \text{on } t$$

holds with the diagonal matrix

$$D_t(p_{l,i}) = \text{diag} (kr_i(\theta_i(p_{l,i}(x_1))), \dots, kr_i(\theta_i(p_{l,i}(x_d)))) .$$

Now we can introduce the bi-form

$$b_{l,i}(w_{l,i}, v_{l,i}) := \sum_{t \in \mathcal{T}_{l,i}} \int_t D_t(w_{l,i}) \nabla w_{l,i} \nabla v_{l,i} dx \quad \forall w_{l,i}, v_{l,i} \in \mathcal{S}_{l,i}$$

for $i = 1, \dots, n$, which can be regarded as an inexact version of $b_i(\cdot, \cdot)$ given in (22), with a quadrature rule involving the quadrature points given by Lemma 3.3.

Altogether, local Kirchhoff transformation of the weak, time-discrete multidomain formulation (24)–(26), discretization of the resulting generalized formulation by finite elements, and local inverse Kirchhoff transformation (37) provides the desired multidomain discretization of the Richards equation. It can be regarded as a finite element discretization of the physical multidomain formulation (24)–(26) with nonstandard quadrature and it reads: Find $p_{l,i} \in \mathcal{S}_{l,i}$, $i = 1, \dots, n$, satisfying

$$(38) \quad \sum_{q \in \mathcal{N}_{l,i}} \theta_i(p_{l,i}(q)) v_{l,i}(q) h_{q,i} + b_{l,i}(p_{l,i}, v_{l,i}) = \ell_{l,i}(v_{l,i}) \quad \forall v_{l,i} \in \mathcal{S}_{l,i}$$

with discrete coupling conditions

$$(39) \quad p_{l,i}(q) = p_{l,j}(q) \quad \forall q \in \mathcal{N}_{l,ij}$$

$$(40) \quad \sum_{q \in \mathcal{N}_{l,ij}} \theta_i(p_{l,i}(q)) \mu_{l,ij}(q) h_{q,i} + b_{l,i}(p_{l,i}, R_{l,ij} \mu_{l,ij}) - \ell_{l,i}(R_{l,ij} \mu_{l,ij}) = \\ - \sum_{q \in \mathcal{N}_{l,ij}} \theta_j(p_{l,j}(q)) \mu_{l,ij}(q) h_{q,j} - b_{l,j}(p_{l,j}, R_{l,ji} \mu_{l,ij}) + \ell_{l,j}(R_{l,ji} \mu_{l,ij}) \\ \forall \mu_{l,ij} \in \mathcal{S}_{l,ij}$$

where (39) and (40) hold for all $i = 1, \dots, n-1$ with $j = i+1$. Analogously as in the continuous case (24)–(26), we obtain $p_l \in \mathcal{S}_l$ by defining $p_l := p_{l,i}$ on Ω_i , $i = 1, \dots, n$, and using (39).

We close this section with the discrete counterpart of Proposition 3.1.

Proposition 3.4. *If $\theta_i : \mathbb{R} \rightarrow \mathbb{R}$ and $kr_i : \theta_i(\mathbb{R}) \rightarrow \mathbb{R}$ are increasing and continuous and $kr_i(\cdot) > 0$, $i = 1, \dots, n$, then the discrete multidomain problem (24)–(26) in physical variables is equivalent to the multidomain problem (38)–(40) in generalized variables. The solutions of the two discrete formulations are related by inverse discrete Kirchhoff transformation (37).*

While (suboptimal) convergence results are available for homogeneous parameter functions [16], related results in the present heterogeneous case are still open.

4. ALGEBRAIC SOLUTION BY NONLINEAR SUBSTRUCTURING METHODS

4.1. Nonlinear Dirichlet–Neumann method. Our version of the Dirichlet–Neumann method is motivated by the checkerboard ansatz in [31, Sec. 1.4.2]. We first apply it to the continuous multidomain problem (28)–(30) and then to the discrete one (32)–(34). In addition, we state some theoretical results.

4.1.1. The continuous case. As in the previous section, we give a formulation of the method for the multidomain problem (28)–(30) with homogeneous boundary conditions on $\partial\Omega$. Therefore, we seek subdomain solutions in the spaces V_i given in (21). We choose a global damping parameter $\vartheta \in (0, 1)$ although one could also choose different ones for each interface. As the initial iterates $u_i^0 \in V_i$, $i = 1, \dots, n$, we take the subdomain solutions from the previous time step. Then, with known iterates $u_i^k \in V_i$ for $k \geq 0$ we obtain the new iterates $u_i^{k+1} \in V_i$ by first solving

$$(41) \quad (n_i M_i(u_i^{k+1}), v_i)_{\Omega_i} + a_i(u_i^{k+1}, v_i) = \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i), \quad \forall i \text{ odd},$$

$$(42) \quad \text{tr}_{\Gamma_{ij}} u_i^{k+1} = \kappa_i \kappa_j^{-1} (\vartheta \text{tr}_{\Gamma_{ji}} u_j^k + (1 - \vartheta) \kappa_j \kappa_i^{-1} \text{tr}_{\Gamma_{ij}} u_i^k) \quad \text{in } \Lambda_{ij}, \\ j = i \pm 1 \in \{1, \dots, n\},$$

and then

$$(43) \quad (n_i M_i(u_i^{k+1}), v_i)_{\Omega_i} + a_i(u_i^{k+1}, v_i) = \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i), \quad \forall i \text{ even},$$

$$(44) \quad (n_i M_i(u_i^{k+1}), R_{ij}\mu)_{\Omega_i} + a_i(u_i^{k+1}, R_{ij}\mu) - \ell_i(R_{ij}\mu) = \\ - (n_j M_j(u_j^{k+1}), R_{ji}\mu)_{\Omega_j} - a_j(u_j^{k+1}, R_{ji}\mu) + \ell_j(R_{ji}\mu) \quad \forall \mu \in \Lambda_{ij}, \\ j = i \pm 1 \in \{1, \dots, n\}.$$

Note that we carry out the damping of Dirichlet values on Γ_{ij} in terms of the transformed variables corresponding to Ω_j . Also, observe that in each iteration step inverse Kirchhoff transformations κ_i^{-1} , κ_j^{-1} have to be evaluated on the interfaces Γ_{ij} . Hence, in contrast to the homogeneous case, the typical ill-conditioning of the Richards equation (cf. Figure 4) is no longer separated from the algebraic solution process.

4.1.2. *Convergence properties.* In 1D the nonlinear Dirichlet–Neumann method for $n = 2$ converges for the stationary Richards equation without gravity in case of sufficiently small damping parameter if we have $c \leq K_{h,i}kr(\cdot) \leq C$ for some positive constants c, C and Lipschitz continuous $K_{h,i} : \Omega_i \rightarrow \mathbb{R}$, see [8, 14]. In higher dimensions convergence can be ensured if the Kirchhoff transformations $\kappa_i, i = 1, 2$, and their inverses are Lipschitz continuous superposition operators in the trace space and certain smallness conditions on their norms are satisfied.

4.1.3. *The discrete case.* Now we turn to the discrete version of (41)–(44), i.e., to our version of the Dirichlet–Neumann method for the fully discrete multidomain problem (32)–(34). Again we choose a global damping parameter $\vartheta \in (0, 1)$ and the subdomain solutions from the previous time step as the initial iterates $u_{l,i}^0 \in \mathcal{S}_{l,i}$, $i = 1, \dots, n$. Then, with known iterates $u_{l,i}^k \in \mathcal{S}_{l,i}$ for $k \geq 0$ we determine the new iterates $u_i^{k+1} \in \mathcal{S}_{l,i}$ by first solving for odd i :

$$(45) \quad \sum_{q \in \mathcal{N}_{l,i}} M_i(u_{l,i}^{k+1}(q)) v_{l,i}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, v_{l,i}) = \ell_{l,i}(v_{l,i}) \quad \forall v_{l,i} \in \mathcal{S}_{l,i}^0$$

$$(46) \quad u_{l,i}^{k+1}(q) = \kappa_i \kappa_j^{-1} (\vartheta u_{l,j}^k(q) + (1 - \vartheta) \kappa_j \kappa_i^{-1} u_{l,i}^k(q)) \quad \forall q \in \mathcal{N}_{l,ij}, \\ j = i \pm 1 \in \{1, \dots, n\},$$

and then for even i :

$$(47) \quad \sum_{q \in \mathcal{N}_{l,i}} M_i(u_{l,i}^{k+1}(q)) v_{l,i}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, v_{l,i}) = \ell_{l,i}(v_{l,i}) \quad \forall v_{l,i} \in \mathcal{S}_{l,i}^0$$

$$(48) \quad \sum_{q \in \mathcal{N}_{l,ij}} M_i(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, R_{l,ij} \mu_{l,ij}) - \ell_{l,i}(R_{l,ij} \mu_{l,ij}) = \\ - \sum_{q \in \mathcal{N}_{l,ij}} M_j(u_{l,j}^{k+1}(q)) \mu_{l,ij}(q) h_{q,j} - a_j(u_{l,j}^{k+1}, R_{l,ji} \mu_{l,ij}) + \ell_{l,j}(R_{l,ji} \mu_{l,ij}) \\ \forall \mu_{l,ij} \in \mathcal{S}_{l,ij}, \quad j = i \pm 1 \in \{1, \dots, n\}.$$

What has been pointed out in Subsection 4.1.2 for the continuous version of the algorithm applies analogously to the discrete version. In particular, *the convergence results carry over to the discrete case with the same constants which entails mesh-independent convergence rates in 1D with $n = 2$* . Indeed, asymptotic mesh-independence of convergence rates and corresponding optimal damping parameters can also be observed in numerical examples in 2D. In the case with two subdomains, the convergence rates improve if extremely different soil types (with $K_{h,1}$ and $K_{h,2}$ differing by some orders of magnitude) are used in Ω_1 and Ω_2 as long as the domain with the higher permeability is chosen as the Neumann domain. See [15] for some numerical studies on this topic.

4.2. **Nonlinear Robin method.** Similarly as for the Dirichlet–Neumann method we now introduce and discuss our version of a nonlinear Robin algorithm applied to the non-overlapping domain decomposition problem (18)–(20) and its discrete counterpart (32)–(34).

4.2.1. *The continuous case.* Robin conditions are linear combinations of Dirichlet and Neumann boundary conditions. Roughly speaking, for a $\gamma > 0$, the Robin method results in an iteration between two subdomains over the sum $\mathbf{v} \cdot \mathbf{n} + \gamma p = \mathbf{v} \cdot \mathbf{n} + \gamma \kappa^{-1} u$ across the interface. This sum, intended to be equal from both neighbouring subdomains, is multiplied with test functions and integrated over the interface, followed by the application of Green's formula to $\mathbf{v} \cdot \mathbf{n}$ on both subdomains meeting at that interface.

On our layered decomposition of Ω this gives rise to the following sequential algorithm that treats the subdomains from the top to the bottom. We choose a relaxation parameter $\gamma_i > 0$ for each Ω_i , $i = 1, \dots, n$, and by $(\cdot, \cdot)_{\Gamma_{ij}}$ abbreviate the L^2 -scalar product on Γ_{ij} , $j = i \pm 1$. As above, we take the subdomain solutions from the previous time step as the initial iterates $u_i^0 \in V_i$, $i = 1, \dots, n$. Then with known iterates $u_i^k \in V_i$ for a $k \geq 0$ we generate the new iterates $u_i^{k+1} \in V_i$ by solving successively for $i = 1, \dots, n$ the Robin subdomain problems

$$(49) \quad (n_i M_i(u_i^{k+1}), v_i)_{\Omega_i} + a_i(u_i^{k+1}, v_i) = \ell_i(v_i) \quad \forall v_i \in H_0^1(\Omega_i)$$

$$(50) \quad (n_i M_i(u_i^{k+1}), R_{ij}\mu)_{\Omega_i} + a_i(u_i^{k+1}, R_{ij}\mu) - \ell_i(R_{ij}\mu) + \gamma_i(\kappa_i^{-1} u_i^{k+1}, \mu)_{\Gamma_{ij}} \\ = -(n_j M_j(u_j^k), R_{ji}\mu)_{\Omega_j} - a_j(u_j^k, R_{ji}\mu) + \ell_j(R_{ji}\mu) + \gamma_j(\kappa_j^{-1} u_j^k, \mu)_{\Gamma_{ij}} \\ \forall \mu \in \Lambda_{ij}, \quad j = i - 1 \in \{1, \dots, n\},$$

$$(51) \quad (n_i M_i(u_i^{k+1}), R_{ij}\mu)_{\Omega_i} + a_i(u_i^{k+1}, R_{ij}\mu) - \ell_i(R_{ij}\mu) + \gamma_i(\kappa_i^{-1} u_i^{k+1}, \mu)_{\Gamma_{ij}} \\ = -(n_j M_j(u_j^k), R_{ji}\mu)_{\Omega_j} - a_j(u_j^k, R_{ji}\mu) + \ell_j(R_{ji}\mu) + \gamma_j(\kappa_j^{-1} u_j^k, \mu)_{\Gamma_{ij}} \\ \forall \mu \in \Lambda_{ij}, \quad j = i + 1 \in \{1, \dots, n\}.$$

As the Dirichlet–Neumann method the Robin method can also be carried out for more general boundary conditions on $\partial\Omega$ like in the hydrological example in Section 5.3. Moreover, since Robin conditions lead to uniquely solvable subdomain problems (see Theorem 4.1 for the conditions and [18] for more details) we do not encounter non-uniqueness issues as they could occur in the Dirichlet–Neumann method. As for the latter in (41)–(44), we could also consider a parallelizable version of the Robin method in which we first treat all subdomains for odd indices i , calculating (49) with (51) also for $j - 1 \in \{1, \dots, n\}$, and then for even indices i , calculating (49) with (50) also for $j + 1 \in \{1, \dots, n\}$.

As for the Dirichlet–Neumann method, the interface conditions require the evaluation of ill-conditioned, inverse Kirchhoff transformations κ_i^{-1} , κ_j^{-1} in each step of the Robin iteration.

4.2.2. *Convergence properties.* We can state the following result for $n = 2$ subdomains and homogeneous Dirichlet boundary conditions [8].

Theorem 4.1. *In 1D and with two subdomains the Robin method (49)–(51) converges if $\gamma_1 = \gamma_2$ and, for $i = 1, 2$, $M_i : \mathbb{R} \rightarrow \mathbb{R}$ are Lipschitz continuous increasing functions and $c \leq K_{h,i} kr(\cdot) \leq C$ while n_i are positive functions in $C(\overline{\Omega}_i)$ and the $K_{h,i} : \Omega_i \rightarrow \mathbb{R}$ are Lipschitz continuous.*

A more general analysis including a convergence result for certain degenerate kr , i.e., with the property $kr(\theta) \rightarrow 0$ for $\theta \rightarrow \theta_m$, is feasible.

4.2.3. *The discrete case.* We discretize (49)–(51) in the way explained in Subsection 3.2 in order to obtain our Robin method for the discrete multidomain problem (32)–(34). The discretization of all terms has appeared above, with the exception of the last integral $(\kappa_j^{-1}, \mu)_{\Gamma_{ij}}$ in the Robin conditions (50) and (51). In accordance with the treatment of the first integrals in (49) over the subdomains, the integrals over the interface Γ_{ij} are discretized via Lagrange interpolation of the integrands. This means, with the weights

$$h_{q,ij} := \int_{\Gamma_{ij}} \lambda_q dx \quad \forall q \in \mathcal{N}_{l,ij}$$

we discretize

$$(\kappa_i^{-1} u_i^{k+1}, \mu)_{\Gamma_{ij}} \quad \text{by} \quad \sum_{q \in \mathcal{N}_{l,ij}} \kappa_i^{-1}(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,ij}.$$

For first-order finite element basis functions λ_q , the $h_{q,ij}$ are positive and the discrete functions

$$u_i^{k+1} \mapsto \sum_{q \in \mathcal{N}_{l,ij}} \kappa_i^{-1}(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,ij}$$

are increasing.

Altogether, again with initial iterates $u_i^0 \in \mathcal{S}_{l,i}$ coming from the solution of the previous time step, we obtain the following scheme for the solution of (32)–(34). With $\gamma_i > 0$ for $i = 1, \dots, n$, and known $u_i^k \in \mathcal{S}_{l,i}$ for a $k \geq 0$ we obtain the new iterates $u_i^{k+1} \in \mathcal{S}_{l,i}$ by solving the discrete Robin subdomain problems

$$(52) \quad \sum_{q \in \mathcal{N}_{l,i}} M_i(u_{l,i}^{k+1}(q)) v_{l,i}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, v_{l,i}) = \ell_{l,i}(v_{l,i}) \quad \forall v_{l,i} \in \mathcal{S}_{l,i}^0$$

$$(53) \quad \sum_{q \in \mathcal{N}_{l,ij}} M_i(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, R_{l,ij} \mu_{l,ij}) - \ell_{l,i}(R_{l,ij} \mu_{l,ij}) \\ + \gamma_i \sum_{q \in \mathcal{N}_{l,ij}} \kappa_i^{-1}(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,ij} = \\ - \sum_{q \in \mathcal{N}_{l,ij}} M_j(u_{l,j}^{k+1}(q)) \mu_{l,ij}(q) h_{q,j} - a_j(u_{l,j}^{k+1}, R_{l,ji} \mu_{l,ij}) + \ell_{l,j}(R_{l,ji} \mu_{l,ij}) \\ + \gamma_i \sum_{q \in \mathcal{N}_{l,ij}} \kappa_j^{-1}(u_{l,j}^{k+1}(q)) \mu_{l,ij}(q) h_{q,ij} \quad \forall \mu_{l,ij} \in \mathcal{S}_{l,ij}, \quad j = i-1 \in \{1, \dots, n\},$$

$$(54) \quad \sum_{q \in \mathcal{N}_{l,ij}} M_i(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,i} + a_i(u_{l,i}^{k+1}, R_{l,ij} \mu_{l,ij}) - \ell_{l,i}(R_{l,ij} \mu_{l,ij}) \\ + \gamma_i \sum_{q \in \mathcal{N}_{l,ij}} \kappa_i^{-1}(u_{l,i}^{k+1}(q)) \mu_{l,ij}(q) h_{q,ij} = \\ - \sum_{q \in \mathcal{N}_{l,ij}} M_j(u_{l,j}^k(q)) \mu_{l,ij}(q) h_{q,j} - a_j(u_{l,j}^k, R_{l,ji} \mu_{l,ij}) + \ell_{l,j}(R_{l,ji} \mu_{l,ij}) \\ + \gamma_i \sum_{q \in \mathcal{N}_{l,ij}} \kappa_j^{-1}(u_{l,j}^k(q)) \mu_{l,ij}(q) h_{q,ij} \quad \forall \mu_{l,ij} \in \mathcal{S}_{l,ij}, \quad j = i+1 \in \{1, \dots, n\}.$$

As in the continuous case the discrete Robin problems are uniquely solvable, even in the case of Brooks–Corey parameter functions that do not satisfy the conditions

in Theorem 4.1, see [8, p. 194]. For the solution theory and the application of multigrid to Robin problems of this kind we refer to [12] and [8, Sec. 3.4].

With the conditions stated in Theorem 4.1 for the continuous case we can also show convergence of the discrete Robin method in 1D [8, Thm. 3.4.30]. Numerically, we also observe convergence in higher space dimensions and even for the Brooks–Corey parameter functions, see Subsection 5.3 or [12]. In contrast to the Dirichlet–Neumann method, we cannot prove mesh-independence. This is no surprise, as the Robin method is known to be mesh-dependent even for linear problems. Instead, the actual asymptotic behaviour of the degenerating convergence rates and corresponding optimal relaxation parameters with respect to the mesh size have been determined theoretically and experimentally for many linear problems, see [27, 30]. We were able to confirm many of those relationships numerically in our nonlinear cases, too. The results of those numerical studies can be found in [15].

5. NUMERICAL EXPERIMENTS

5.1. Discretization error. To numerically assess the spatial discretization error of our multidomain discretization, we construct a model problem (14)–(16) on $n = 2$ subdomains with known closed-form solution. We omit additional source terms and the gravity term for simplicity, i.e., we set $f_i = n_i \theta_i(\bar{p}_i)$, $f_{ij} = f_{ji} = 0$, and $n_i = \tau_k = 1$. In order to capture typical challenges of the given problem, we consider a solution with saturated and unsaturated parts in each subdomain, and such that the saturation front, i.e., the border between these parts, crosses the interface.

The computational domain $\Omega = (-1, 1)^2$ is divided into two subdomains $\Omega_1 = (-1, 1) \times (0, 1)$ and $\Omega_2 = (-1, 1) \times (-1, 0)$, which meet at the interface $\Gamma = \overline{\Omega_1} \cap \overline{\Omega_2} = (-1, 1) \times \{0\}$ (meters are used as the unit of length). On Ω we make the ansatz

$$p(x, y) = p_i(x, y) \quad \text{in } \Omega_i,$$

and

$$p_i(x, y) = \omega(x) + a g_i(y) K_{h,i}^{-1} kr_i(\theta_i(\omega(x)))^{-1}, \quad i = 1, 2.$$

Here, $\omega, g_i \in C^1(\mathbb{R})$ are functions to be specified below, $K_{h,1}, K_{h,2}$ are the hydraulic conductivities in the subdomains, θ_i and kr_i are the parameter functions (2) and (3), respectively, and $a \in \mathbb{R}$ is a scaling parameter.

To obtain the desired behaviour we set

$$\omega(x) = \begin{cases} p_{b,1} - (p_{b,2} - p_{b,1}) \left(\frac{x - \xi_1}{\Delta\xi}\right)^2 & \text{if } x \leq \xi_1, \\ p_{b,1} + (p_{b,2} - p_{b,1}) \left(\frac{x - \xi_2}{\Delta\xi} - 1\right)^2 \left(\frac{x - \xi_2}{\Delta\xi} + 1\right)^2 & \text{if } \xi_1 \leq x \leq \xi_2, \\ p_{b,2} + (p_{b,2} - p_{b,1}) \left(\frac{x - \xi_2}{\Delta\xi}\right)^2 & \text{if } \xi_2 \leq x, \end{cases}$$

where $-1 < \xi_1 < \xi_2 < 1$ are two parameters and $\Delta\xi = \xi_2 - \xi_1$. We assume that $p_{b,2} > p_{b,1}$. For g_i we select

$$g_1(y) = \sin(2\pi y) \quad \text{and} \quad g_2(y) = 2\pi y.$$

Note that $g_1(0) = g_2(0)$, and hence p is continuous across Γ . Also, $g_1'(0) = g_2'(0)$, and hence the flux continuity condition (16)

$$K_{h,1} kr_1(\theta_1(p_1(x, 0))) \frac{\partial}{\partial y} p_1(x, 0) = K_{h,2} kr_2(\theta_2(p_2(x, 0))) \frac{\partial}{\partial y} p_2(x, 0)$$

	n	θ_m	θ_M	λ	p_b [m]	K_h [m/s]
sand	0.437	0.0458	1	0.694	-0.0726	$6.54 \cdot 10^{-5}$
loam	0.463	0.0583	1	0.252	-0.1115	$3.67 \cdot 10^{-6}$

TABLE 1. Soil parameters

holds as well. We check that the saturation front crosses the interface. Recall from (2) that $(x, y) \in \Omega_i \cup \Gamma$ is on the saturation front if $p(x, y) = p_{b,i}$ and $p(\tilde{x}, \tilde{y}) < p_{b,i}$ for some (\tilde{x}, \tilde{y}) in every neighbourhood of (x, y) in $\Omega_i \cup \Gamma$. By construction we have $p(x, 0) = \omega(x)$ and $\omega(x) = p_{b,i}$ if $x = \xi_i$. From the definition of ω we see that $\omega(x) < p_{b,1}$ if $x < \xi_1$, $\omega(x) > p_{b,2}$ if $x > \xi_2$, and $p_{b,1} < \omega(x) < p_{b,2}$ for $\xi_1 < x < \xi_2$. Hence the saturation front intersects Γ on at least $[\xi_1, \xi_2] \times \{0\}$.

We finally discuss the smoothness of p . Note that the derivative

$$(kr_i(\theta_i(\omega(x))))' = \begin{cases} -(2 + 3\lambda_i) \left(\frac{\omega(x)}{p_{b,i}}\right)^{-3-3\lambda_i} \frac{\omega'(x)}{p_{b,i}} & \text{if } x \leq \xi_i, \\ 0 & \text{if } x > \xi_i, \end{cases}$$

is continuous, since $\omega(\xi_i) = p_{b,i}$ and $\omega'(\xi_i) = 0$, and piecewise differentiable with respect to x . From this we conclude that

$$\frac{\partial}{\partial x} p_i(x, y) = \omega'(x) - g_i(y) K_{h,i}^{-1} kr_i(\theta_i(\omega(x)))^{-2} (kr_i(\theta_i(\omega(x))))'$$

is also continuous and piecewise differentiable with respect to x and y . Continuity and piecewise differentiability of $\frac{\partial}{\partial y} p_i(x, y)$ with respect to x and y is also easy to verify. Hence $p_i \in H^2(\Omega_i)$ for both subdomains. Since the p_i are continuous across Γ , we get $p \in H^1(\Omega)$ for the entire domain. However, since the normal derivative jumps across Γ , we get $p \notin H^2(\Omega)$. Analogously, since $kr_i(\theta_i(p_i)) \nabla p_i$ is continuous and the classical derivatives exist piecewise, the weak derivatives of $kr_i(\theta_i(p_i)) \nabla p_i$ coincide with the strong ones wherever those exist. Hence the right hand sides $f_i \in L^2(\Omega_i)$ can be computed pointwise in Ω_i as

$$(55) \quad f_i := \theta_i(p_i) - \operatorname{div} (K_{h,i} kr_i(\theta_i(p_i)) \nabla p_i), \quad i = 1, 2.$$

Denote by p_∂ the trace of p on $\partial\Omega$. Then, p is a solution of our benchmark problem

$$\begin{aligned} \theta_i(p_i) - \operatorname{div} (K_{h,i} kr_i(\theta_i(p_i)) \nabla p_i) &= f_i & \text{in } \Omega_i, \quad i = 1, 2, \\ p &= p_\partial & \text{on } \partial\Omega. \end{aligned}$$

We picked soil parameters corresponding to sand in Ω_1 and loam in Ω_2 as given in [32] and listed in Table 1. We set $\xi_1 = -0.4$, $\xi_2 = 0.4$, and the scaling parameter $a = 10^{-7}$. With these data, Figure 6 shows the solution p on the left with the range $[-0.178 \text{ m}, -0.023 \text{ m}]$ on Ω_1 and $[-0.248 \text{ m}, -0.051 \text{ m}]$ on Ω_2 . The size of the unsaturated regimes and a comparison of the range of p with the bubbling pressures in Table 1 make clear that we have chosen a nonlinear problem in which the linear, i.e., saturated regimes do not seem to dominate. The corresponding source term f with $f|_{\Omega_i} = f_i$ is depicted on the right in Figure 6. Note that this is a challenging numerical problem, as the source term f is not continuous on the subdomains and features extreme ‘‘ridges’’, which are difficult to integrate numerically. We use a

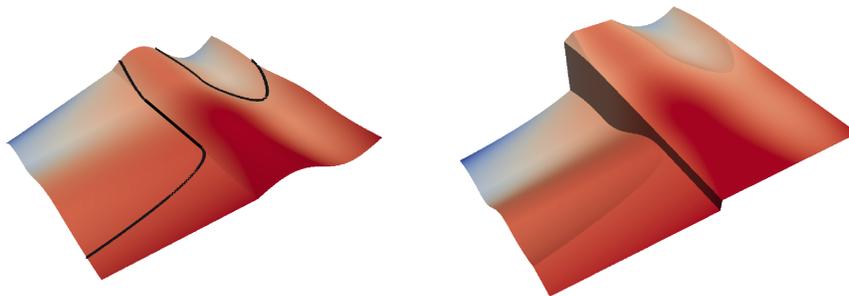


FIGURE 6. Left: solution p of the benchmark problem (black lines: saturation front). Right: corresponding source term f . Subdomain Ω_2 is on the left.

composite quadrature rule obtained by uniformly refining each element three times, and using a fifth-order quadrature on each subelement.

We discretized the domain Ω with a uniform triangle grid on 3×3 vertices, and constructed a sequence of test grids by up to 10 steps of uniform refinement. We use the DUNE libraries¹ [5] for the implementation, and DUNE-GRID-GLUE [6] to handle the grid coupling. For each grid we solved the problem using the Dirichlet–Neumann method of Section 4.1. We had the method iterate until the relative size of the correction in p measured in the energy norm dropped below 10^{-13} . This is shortly before rounding errors prevent further progress. The subdomain problems were solved in the generalized pressure u_i , $i = 1, 2$, by uniform multigrid iterating until the relative norm of the correction dropped below 10^{-12} .

We first consider the error of the multidomain discretization (32)–(34) in the generalized pressure u . For this note that the function $u : \Omega \rightarrow \mathbb{R}$ with $u|_{\Omega_i} = u_i$ defined by

$$u_i := \kappa_i(p_i) \quad \text{in } \Omega_i$$

is discontinuous across Γ and solves the generalized multidomain problem (18)–(20) with the same source term f as given in (55). We compute discrete solutions $u_{l,i}$, $i = 1, 2$, and measure the error against the analytical solution u in the L^2 -norm on Ω and, since u is discontinuous across Γ , in the broken H^1 -norm on $\Omega_1 \cup \Omega_2$. For the latter, the gradient of u is needed on the subdomains. Using the chain rule (7), i.e., $\nabla u_i = \kappa_i'(\theta_i(\tilde{p}_i)) \nabla \tilde{p}_i$, which is valid for $p_i \in H^1(\Omega_i)$, see [29], we can reuse the formulas for p_i and apply a quadrature rule to compute the integrals. We use the composite quadrature rule with three steps of refinement and a fifth-order rule on each subelement. The result can be seen in Figure 7, left. Both the L^2 - and the H^1 -error of the discrete generalized multidomain formulation (32)–(34) show optimal convergence orders h^2 and h , respectively, but only after about 5–7 refinement steps. The reason is that the source terms f_i contain discontinuities and steep gradients that can only be resolved on sufficiently fine grids.

Now $p_l \in \mathcal{S}_l$ is obtained from $u_{l,i}$, $i = 1, 2$ by inverse discrete Kirchhoff transformation (37). We computed the error $\|p - p_l\|$ in the L^2 -norm and in the H^1 -norm. The result can be seen in Figure 7, right. Again, optimal convergence orders are obtained once the grid is sufficiently fine to resolve the features of the right hand

¹www.dune-project.org

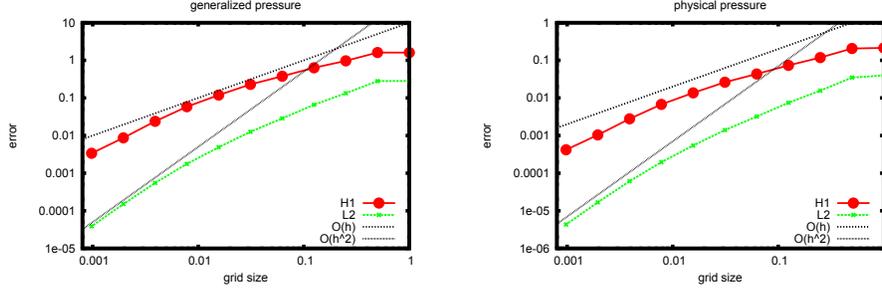


FIGURE 7. Discretization errors as a function of grid size h . Left: generalized pressure u . Right: physical pressure p

side f . In fact, the error reduction curves for p and for u look quite similar in the graphics and seem to differ just by approximately the factor 10, whose inverse is about the order of magnitude of the bubbling pressures in Table 1. This reflects the fact that in our concrete calculations we scale u by the bubbling pressure to obtain an adimensional variable. Thus the inverse Kirchhoff transformations $\kappa_i^{-1} : \mathbb{R} \rightarrow \mathbb{R}$, $i = 1, 2$, seem to have hardly an influence on the discretization error in this case, which suggests that their mean Lipschitz constants on the images of u_i are of order of magnitude 1. In particular, we do not observe saturation of L^2 - or increase of H^1 -discretization errors for the physical pressure p here in contrast to the experimental discretization result reported in [16, Sec. 5.1]. The reason is that the potentially ill-conditioned inverse discrete Kirchhoff transformations amplify round-off errors at most by a factor of $\max_{x \in \Omega, i=1,2} (\kappa_i^{-1})'(u_i(x)) \approx 39$.

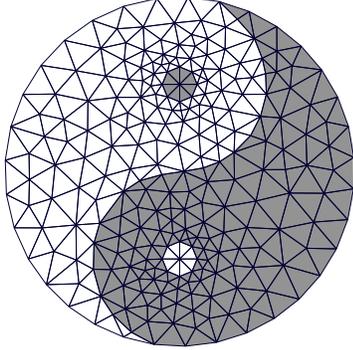


FIGURE 8. Yin-Yang domain and grid

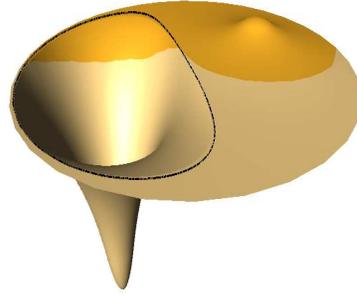


FIGURE 9. Typical shape of solution with saturation front

5.2. Robustness of substructuring with respect to Brooks–Corey parameters. We consider the multidomain formulation (14)–(16) for the Richards equation on the Yin–Yang domain Ω with radius one, as depicted in Figure 8. As in the previous subsection we omit the gravity term, but now we include source terms, i.e., we have $f_i = \bar{f}_i + n_i \theta_i(\bar{p}_i)$ and $f_{ij} = f_{ji} = 0$. The white subdomain together with the grey circle B_1 and the grey subdomain with the white circle B_2 are denoted by Ω_1

and Ω_2 , respectively. We select $\bar{f}_1|_{B_1} = 5 \cdot 10^{-5} \text{ s}^{-1}$, $\bar{f}_2|_{B_2} = -10^{-1} \text{ s}^{-1}$, and zero elsewhere, so that B_1 and B_2 can be regarded as a source and a sink, respectively. The reference soil parameters corresponding to sand are given in Table 1, and are chosen to be the same in both domains. The problem is discretized as described in Section 3 with the time step size $\tau_1 = 1 \text{ s}$ and a spatial mesh \mathcal{T}_4 resulting from four uniform refinements of the initial triangulation \mathcal{T}_0 depicted in Figure 8. We consider the algebraic solution of the spatial problem occurring in the first time step, starting from the initial condition $p_0(x) = 0.45(|x| - 1)$.

In contrast to homogeneous Brooks–Corey parameters, ill-conditioning and algebraic solution are no longer separated by the discretization in case of different soil parameters λ_i or $p_{b,i}$ on both sides of Γ , because the evaluation of the inverse Kirchhoff transformation appears in the transmission conditions (15). Nevertheless, in [15] we have presented nontrivial examples for which the nonlinear versions of both the Dirichlet–Neumann and the Robin methods (45)–(48) and (52)–(54) show similar mesh dependence as their counterparts in the linear self-adjoint case. Here we concentrate on robustness of these methods with respect to jumps in the pore size distributions λ_i and the bubbling pressures $p_{b,i}$ in Ω_i , $i = 1, 2$. To this end, we consider variations

$$10^{-2} \leq \lambda_2 \leq 1, \quad 10^{-2} \leq -p_{b,2} \leq 1$$

in Ω_2 , the domain with the sink. The ranges are chosen to cover all physically relevant parameters, compare [32, Table 5.3.2]. In Ω_1 we always keep the reference values λ_1 and $p_{b,1}$ of sand. See Figure 9 for how a solution typically looks like in these cases.

Convergence rate measurements and termination criteria are implemented using the subdomain energy norms given by

$$\|v_i\|_i^2 = a_i(v_i, v_i) \quad \forall v_i \in V_i \quad i = 1, 2,$$

on the spaces defined in (21). The local problems are solved by the monotone multigrid method of [16] until for the iterates v_i^j , $i = 1, 2$, $j = 1, 2, \dots$, the relative norm of the correction $\|v_i^{j+1} - v_i^j\|_i / \|v_i^j\|_i$ dropped below 10^{-12} . For convergence rate estimates of the substructuring algorithms we used the 2-norm on $V_1 \oplus V_2$ defined by

$$\|v\| = (\|v_1\|_1^2 + \|v_2\|_2^2)^{1/2} \quad \forall v = (v_1, v_2) \in V_1 \oplus V_2.$$

The convergence rate of the substructuring iteration with the iterates $u^\nu \in \mathcal{S}_{4,1} \otimes \mathcal{S}_{4,2}$, $\nu = 1, 2, \dots$, in the generalized pressure is approximated by taking the arithmetic mean of the geometric means

$$\rho_\nu = \sqrt[\nu]{\frac{\|u^{\nu+1} - u^\nu\|}{\|u^1 - u^0\|}}$$

for $\nu = \lfloor \nu_0/3 \rfloor, \dots, \nu_0$ with ν_0 chosen such that $\|u^{\nu_0+1} - u^{\nu_0}\| \leq 10^{-8}$. The initial iterate is chosen as the Lagrange interpolated discrete Kirchhoff–transformed initial condition p_0 .

5.2.1. Dirichlet–Neumann method. We first consider the Dirichlet–Neumann iteration as described in Subsection 4.1. A Dirichlet problem on Ω_1 and a corresponding Neumann problem on Ω_2 is solved in each iteration step. For each set of parameters, an approximation of the optimal damping parameter ϑ is determined by a

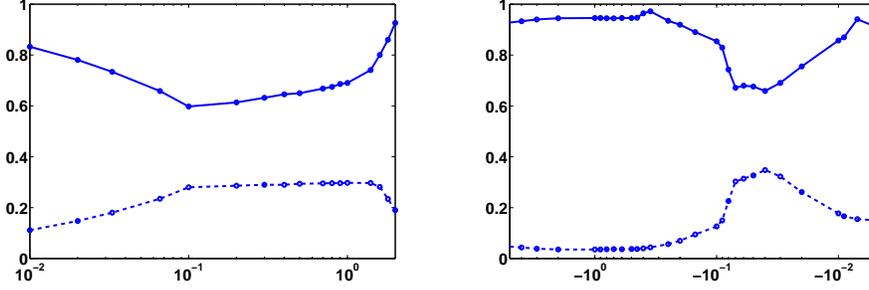


FIGURE 10. Convergence rates ρ_ν (solid line) and damping factor ϑ (dashed line) of the Dirichlet–Neumann method over pore size distribution factor λ_2 (left) and bubbling pressure $p_{b,2}$ (right) in subdomain Ω_2

heuristic strategy. Concretely, we applied the method first for damping parameters coming from a coarse sampling of the parameter interval $[0, 1]$ and then from a finer sampling of the region where good convergence rates have been observed.

The left picture of Figure 10 shows the convergence rate ρ as a function of the pore size distribution λ_2 in Ω_2 (solid line) together with the associated damping parameter ϑ (dashed line). It turns out that the method converges for the whole range of λ_2 . While $\rho \leq 0.75$ holds for $10^{-1} \leq \lambda_2 \leq 1$, convergence rates deteriorate outside of this interval.

The right picture of Figure 10 indicates that the convergence speed of the Dirichlet–Neumann method is more sensitive to variations of the bubbling pressure $p_{b,2}$ in Ω_2 . Outside of a window of $3 \cdot 10^{-2} \leq -p_{b,2} \leq 7 \cdot 10^{-2}$, where $\rho < 0.7$ is reached, the convergence rate quickly deteriorates up to values of $\rho > 0.9$ (solid line). Interestingly, in both pictures, ρ and ϑ seem to always add up to one. This mirrors a known fact from the linear theory which can be easily verified in 1D, see, e.g., [31, p. 12].

5.2.2. Robin method. For a comparison we now consider the Robin method presented in Subsection 4.2 where we prescribe the acceleration parameter $\gamma = \gamma_1 = \gamma_2$. Similarly as for the Dirichlet–Neumann method we determine an approximation of the optimal γ by a heuristic strategy based on coarse sampling of the relevant parameter interval followed by a finer sampling of regions exhibiting good convergence rates.

The dependence of the convergence rate ρ of the Robin iteration on the pore size distribution factor λ_2 is depicted in the left picture of Figure 11. Corresponding optimal acceleration parameters γ are in the interval $[10^{-3}, 10^{-2}]$ and generally tend to be bigger the smaller ρ is.

It turns out that $\rho < 0.5$ holds throughout the whole range of λ_2 . Moreover, the right picture in Figure 11 indicates a similar robustness with respect to variation of the bubbling pressure $p_{b,2}$ for a similar range of parameters $\gamma \approx 0.5 \cdot 10^{-2}$. Theoretical justification of these promising results will be subject of future research. Since the Robin method shows a better behaviour than the Dirichlet–Neumann method in these parameter studies we apply the Robin method in the hydrological example to which we turn now.

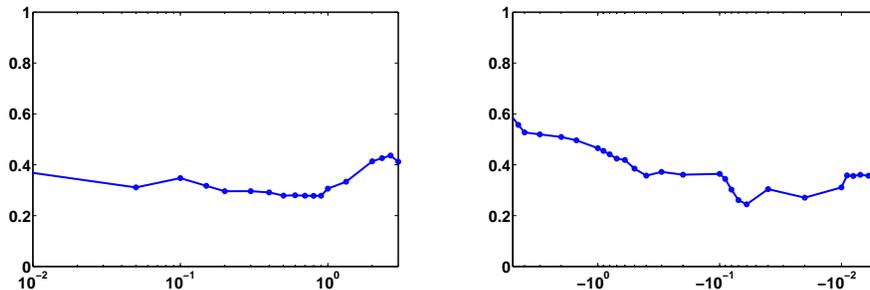


FIGURE 11. Convergence rates ρ_ν of the Robin method over pore size distribution factor λ_2 (left) and bubbling pressure $p_{b,2}$ (right) in subdomain Ω_2

5.3. Seepage of ground and surface water. The final numerical example contains a hydrologically realistic setting with four different soil layers and including surface water. We first describe the surface water model and its coupling to the Richards equation before giving the details and results of the concrete example. For an exhaustive exposition of the example see [8, Sec. 4.3].

5.3.1. *Coupling of the Richards equation with surface water: a compartment model.* The principle of the reservoir model as depicted in Figure 12 is the following. First, we assume that the surface water is non-moving and that for (almost) all times $t > 0$, the water table is horizontal and uniquely determined by a real number $h(t)$, the height of water with respect to the lowest point of the lake. If the geometry that surrounds the lake is given, knowing $h(t)$ is equivalent to knowing the volume $V(h(t))$ and the mass $m(h(t))$ of surface water in the reservoir.

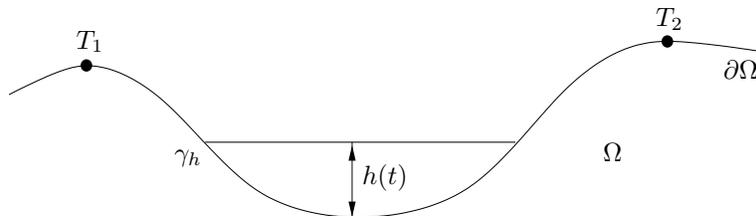


FIGURE 12. Flow across γ_h between T_1 and T_2 affects lake height $h(t)$

The continuous case. We assume that by its hydrostatic pressure, the height $h(t)$ of surface water imposes a Dirichlet boundary condition $\tilde{p}_D = h(t)$ for the Richards equation (recall that the physical pressure in (1) is already measured in meters of a water column). On the other hand we assume mass conservation, i.e., the flow of water out of or into the ground affects the height or volume or mass of the lake corresponding to its mass gain or loss, respectively. With the water density ϱ this condition on the normal flux $\tilde{\mathbf{v}} \cdot \mathbf{n}$ out of Ω can be formulated as

$$(56) \quad \frac{d}{dt}m(h(t)) = \varrho \frac{d}{dt}V(h(t)) = \varrho \int_{\gamma_h} \tilde{\mathbf{v}}(x, t) \cdot \mathbf{n} \, d\sigma(x).$$

Here, γ_h is the portion of $\partial\Omega$ between the top points T_1 and T_2 in Figure 12, on which the flow of water into or out of the ground affects the lake height. Altogether, the compartment model with the mass conservation (56) and the Dirichlet condition $\tilde{p}_D = h(t)$ as coupling conditions result in the coupling of an ordinary and a partial differential equation. For another example of that kind we refer to [18].

The time-discrete case. While we use an implicit–explicit discretization for the Richards equation, we choose an explicit one for the surface water, i.e. for the ODE (56). Concretely, the time-discrete volume V of the lake at the time t_k is

$$(57) \quad V := \bar{V} + \int_{\gamma_h} \bar{\mathbf{v}}(x) \cdot \mathbf{n} \, d\sigma(x),$$

where \bar{V} is the time-discrete volume of the lake at the time t_{k-1} and $\bar{\mathbf{v}}$ is the time-discrete water flux for the time t_{k-1} . The time-discrete solution for the Richards equation at the time t_k is then given by the lake height that corresponds to V and serves as the Dirichlet boundary value p_D .

An implicit time discretization of (56) would require an additional solution technique for the coupled PDE and ODE. See [5, 13] for iteration methods of Dirichlet–Neumann- or Robin–Neumann-type applied to coupling problems of this kind.

A weak formulation of (57) can only be given approximately, since

$$(58) \quad \int_{\gamma_h} \bar{\mathbf{v}}(x) \cdot \mathbf{n} \, d\sigma(x) = \int_{\partial\Omega} \bar{\mathbf{v}}(x) \cdot \mathbf{n} \cdot \mu \, d\sigma(x)$$

might only hold for μ on $\partial\Omega$ with $\mu = 1$ on γ_h and $\mu = 0$ on $\partial\Omega \setminus \gamma_h$. This, however, does not define an element of $H^{1/2}(\partial\Omega)$ so that we cannot apply an extension operator to μ (as in (30)) in order to obtain a weak form of the right hand side of (58) by Green’s formula. Since this problem occurs in the fully discrete case, too, we need to approximate μ by a suitable element of $H^{1/2}(\partial\Omega)$.

The fully discrete case. For the finite element discretization of the time-discretized mass conservation (57) we approximate the integral as follows. In view of Figure 12, we consider the set \mathcal{N}_h of all vertices of the grid lying on γ_h between T_1 and T_2 (or possibly equal to these points). We define the finite element function μ_h on $\partial\Omega$ that is 1 in each of the nodes in \mathcal{N}_h and 0 in all other vertices of $\partial\Omega$. The finite element function v_h shall be its trivial extension on Ω given by $v_h(q) = 0$ for all $q \in \mathcal{N}$. The discretization of (57) is motivated by applying Green’s formula to the right hand side of (58) with μ_h instead of μ and discretizing the obtained weak form just as (34) was obtained from (30). Therefore, with the discrete volume \bar{V}_l from the former time step we define the new discrete volume by

$$V_l := \bar{V}_l - \sum_{q \in \mathcal{N}_h} M_1(u_{l,1}(q)) \mu_h(q) h_{q,1} - a_1(u_{l,1}, v_h) + \ell_{l,1}(v_h),$$

where $u_{l,1}$ is the solution of (32)–(34) for the top subdomain at the time t_{k-1} .

For simplicity we assume that the geometry of the lake can be approximated by a circle line. Then, in order to determine the height h_l of the lake approximatively from the obtained volume V_l at the time t_k we use numerical integration of the circle segment representing V_l .

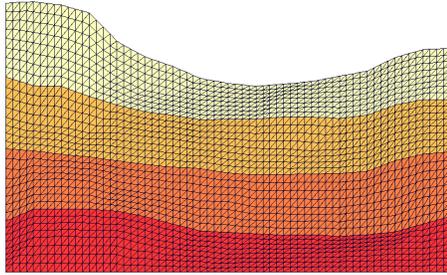


FIGURE 13. Fine grid,
four soil layers as in Table 2

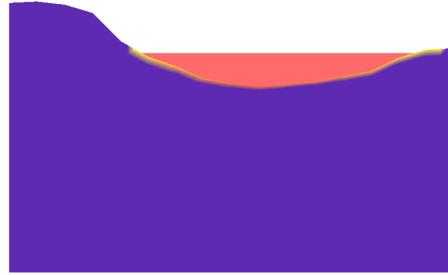


FIGURE 14. Initial condition:
dry soil and surface water

5.3.2. *Setting and numerical results.* With regard to our concrete example, Figure 13 shows the domain $\Omega \subset \mathbb{R}^2$ decomposed into the four subdomains Ω_1 , Ω_2 , Ω_3 and Ω_4 from the top to the bottom. The width of the domain is 2 m, and the height from the bottom to the highest point of Ω is approximately 1.214 m. We apply the Brooks–Corey parameter functions (2) and (3). We choose the soil parameters of sand, loamy sand, sandy loam and loam as given in Table 2 (compare [32, Tables 5.3.2 and 5.5.5]). Figure 13 shows the finest grid (with the mesh size $h = 0.038$) which we obtain on the third refinement level with 585 nodes in each subdomain.

Ω_i	n_i	$\theta_{m,i}$	$\theta_{M,i}$	λ_i	$p_{b,i}$ [m]	$K_{h,i}$ [m/s]
$i = 1$ (sand)	0.437	0.0458	1	0.694	−0.0726	$6.54 \cdot 10^{-5}$
$i = 2$ (loamy sand)	0.437	0.0801	1	0.553	−0.0869	$1.66 \cdot 10^{-5}$
$i = 3$ (sandy loam)	0.453	0.0905	1	0.378	−0.1466	$6.06 \cdot 10^{-6}$
$i = 4$ (loam)	0.463	0.0583	1	0.252	−0.1115	$3.67 \cdot 10^{-6}$

TABLE 2. Soil parameters in the four layers from top to bottom

As the initial condition depicted in Figure 14 in a colourplot we choose $p = -10$ (meters of a water column) corresponding to a practically dry soil in Ω except for the nodes on the top boundary which are covered by surface water (red in Figure 14), where a hydrostatic pressure from the lake is given. The height of the lake at the time $t = 0$ s is 0.1686 m, the radius of the circle line by which we approximate γ_h is $r = 1.2$ m.

The hydrological situation on the banks of the lake is described by Signorini-type boundary conditions on the rest of the top boundary of Ω , see Subsection 2.2. In the time evolution we impose a constant inflow of $-\mathbf{v} \cdot \mathbf{n} = 3 \cdot 10^{-4}$ m/s across the lower half of the left boundary of Ω_1 . On the rest of the left boundary of Ω as well as on the right and the bottom part of $\partial\Omega$ homogeneous Neumann boundary conditions $\mathbf{v} \cdot \mathbf{n} = 0$ are assumed for all time steps. For the time evolution we choose the constant time step size $\tau = 10$ s.

Based on the numerical results in Subsection 5.2 we choose the Robin method (52)–(54) for the solution of the spatial problems. We use the acceleration parameter $\gamma = 10^{-4}$ suggested by numerical experiments for all subdomains and at all time steps. In addition, note that the Dirichlet–Neumann method would encounter non-uniqueness issues with the Neumann data imposed on $\partial\Omega$. As in Subsections 5.1 and 5.2, the inner solver we applied to the homogeneous problem on each subdomain Ω_i for $i = 1, 2, 3, 4$, is the monotone multigrid method. We stopped this iteration when the relative size of the corrections of u_i in the energy norm dropped below 10^{-12} .

In each time step we determine the discrete physical pressure p in the finite element space \mathcal{S} , which is given by the mesh and the boundary conditions, up to an accuracy of 1% in the (semi-)norm

$$\|p\| := \left(\sum_{i=1}^4 a_i(p_i, p_i) \right)^{1/2}, \quad p \in \mathcal{S}, \quad p_i := p|_{\Omega_i}, \quad i = 1, 2, 3, 4.$$

Intensive numerical tests show that we may assume a maximal convergence rate of $\rho = 0.95$ for the Robin iteration in the whole evolution. Assuming $\|p\| \approx \|p^k\|$ for the Robin iterates p^k , $k \geq 0$, is justified since the initial iterate is the solution of the previous time step and the time step size $\tau = 10$ s is moderate. Therefore, the stopping criterion

$$(59) \quad \frac{\|p^{k+1} - p^k\|}{\|p^k\|} < 0.0005$$

gives the desired accuracy

$$(60) \quad \frac{\|p - p^k\|}{\|p\|} < \frac{1}{1 - 0.95} \cdot 0.0005 = 0.01.$$

Figure 19 shows the number of Robin iterations needed per time step with this stopping criterion for the evolution displayed in Figures 15–18. Mostly we obtain iteration numbers below 15 except for the first few time steps, in which the evolution is considerably faster as the saturation front crosses the first layer of sand with a relatively big hydraulic conductivity $K_{h,1}$. Here the solution from the previous time step as the initial iterate for the next time step is further away from the next solution than later in the evolution when the saturation front is slower as it crosses layers with smaller hydraulic conductivity. As Figures 15–18 show, the evolution contains emptying of the lake, a dry lake and again rising of the water level until at the end a fully saturated soil is reached for which the boundary value problems for the Richards equation become linear Darcy problems. The range of p varies from -10 (blue in the colour plots) over $p \approx 0$ in the yellow regions to $p \approx 1$ (orange) until $p \approx 2$ in the red regions.

Recall from Subsection 3.2.2 that we use a time-explicit upwind discretization of the convective (gravitational) term. We do not see instabilities in the numerical solutions due to this term. Our time step size $\tau = 10$ s obeys the CFL condition for linear cases which requires

$$\tau < \frac{n_1}{K_{h,1} \left(3 + \frac{2}{\lambda_1} \right)} h = 43.2 \text{ s}$$

with our data, compare [8, Sec. 4.2.1].

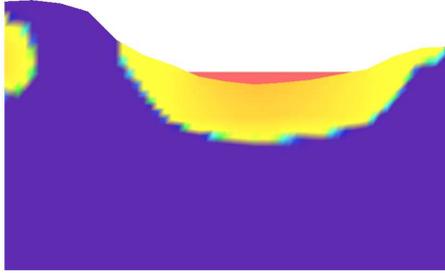
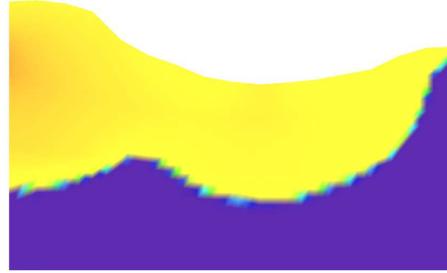
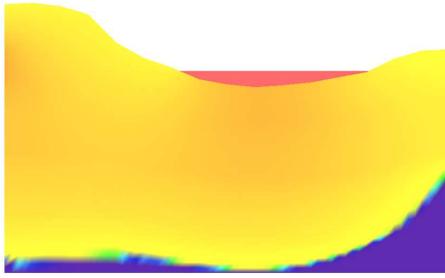
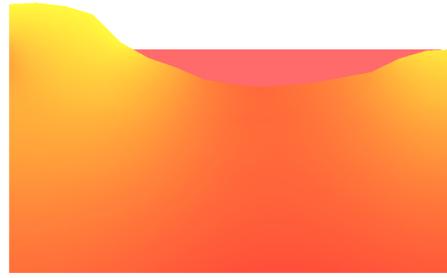
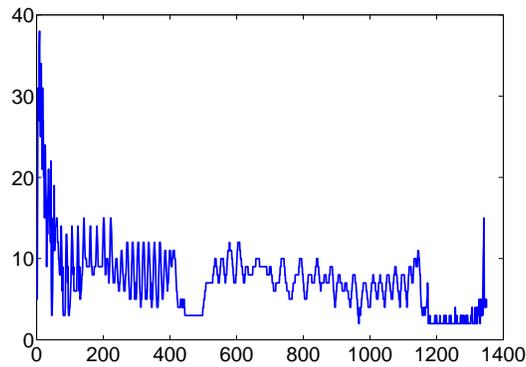
FIGURE 15. $t = 200$ FIGURE 16. $t = 4000$ FIGURE 17. $t = 10000$ FIGURE 18. $t = 13420$ 

FIGURE 19. Number of iterations per time step of the Robin method with stopping criterion (59) for a relative accuracy (60) of 1%

Altogether, the nature of our example and the numerical results we obtained demonstrate that the solution method we propose for the Richards equation in heterogeneous layered soil with surface water can be successfully applied to a realistic hydrological model problem.

ACKNOWLEDGEMENTS

The authors would like to thank Jan Schreiber for his assistance in the production of the parameter studies.

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